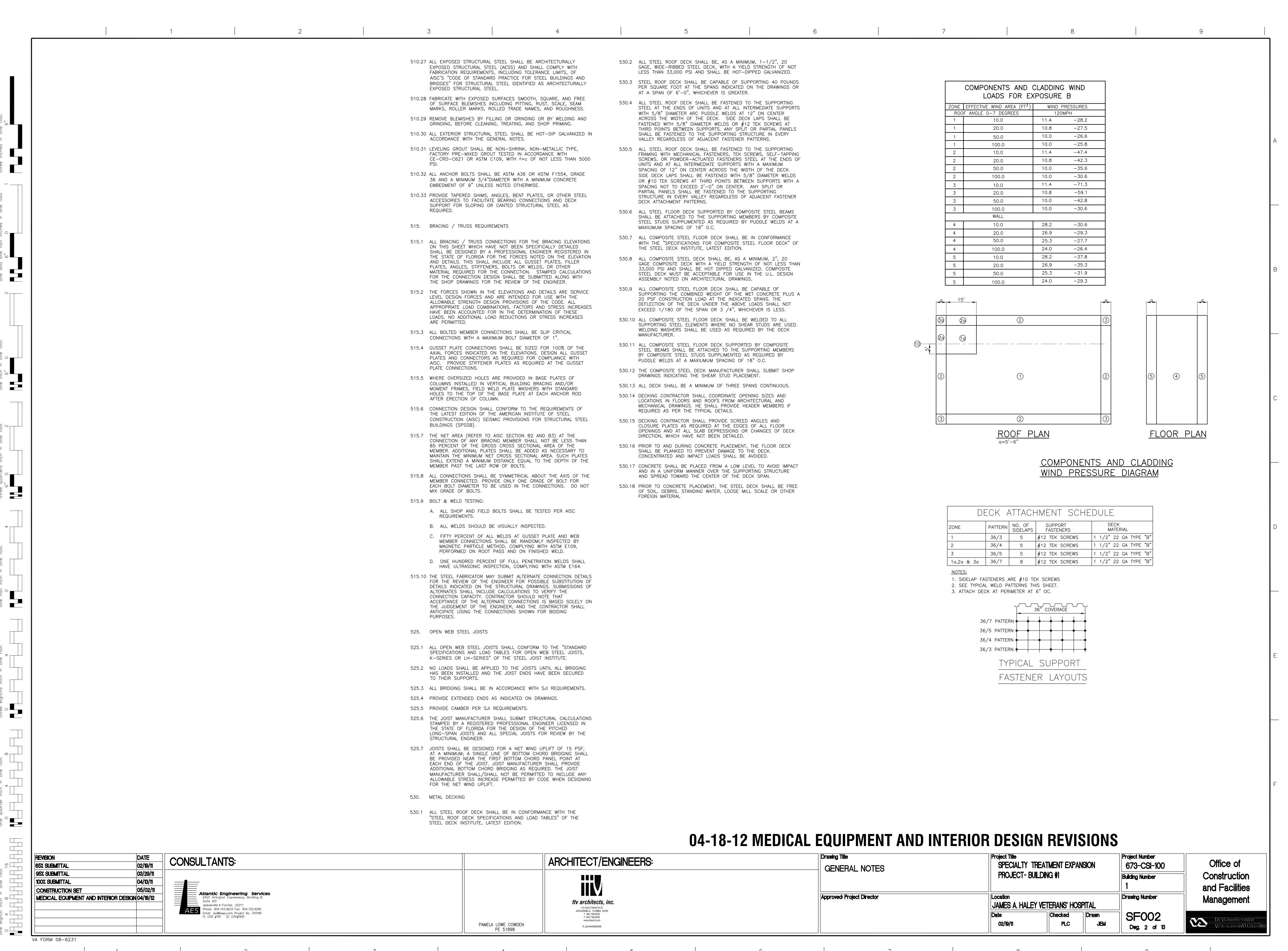
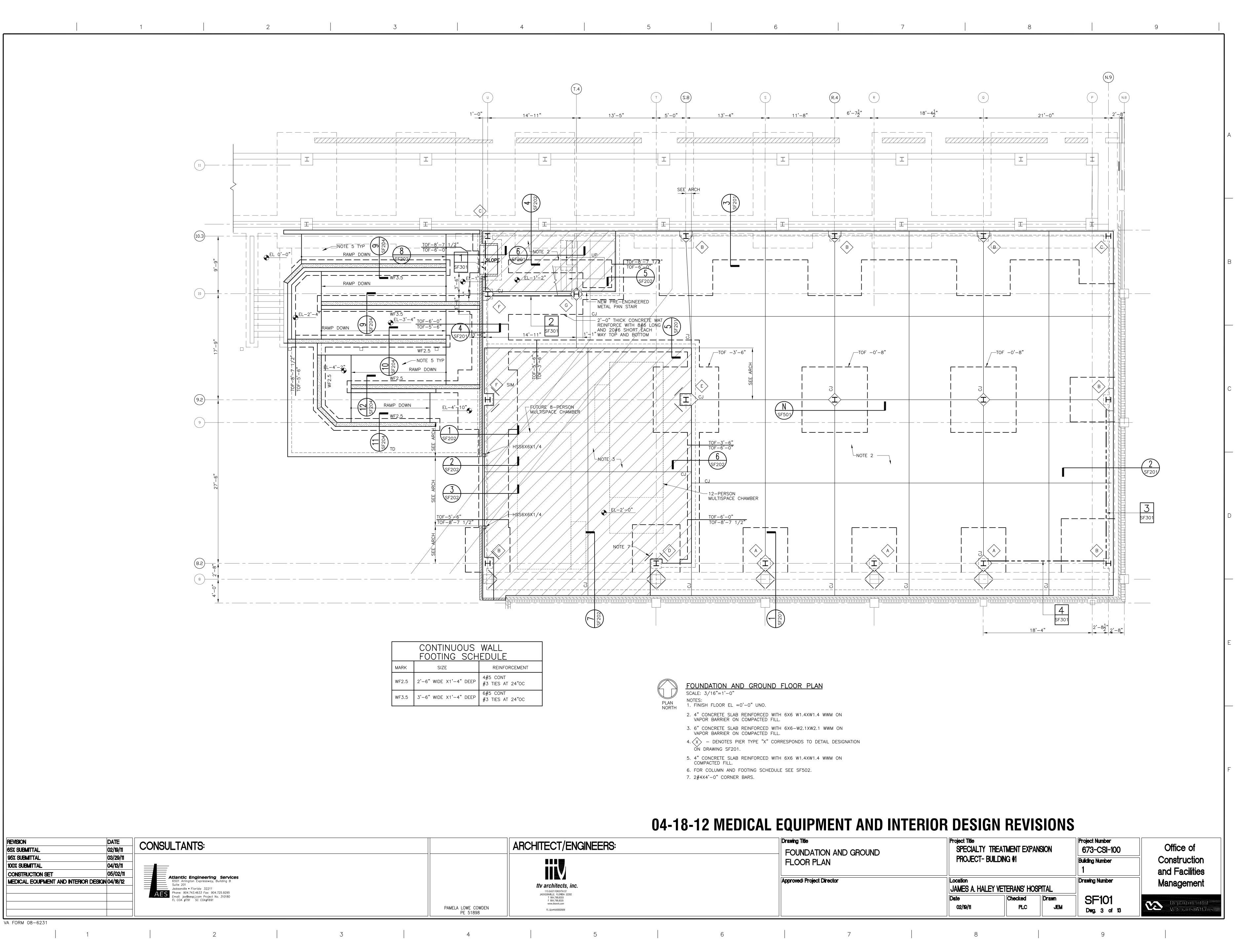
510.5 PROVIDE THE FOLLOWING BOLTED JOINT TYPES UNLESS OTHERWISE 350.7 THE "KWIK-BOLT II EXPANSION ANCHORS" STUD SHALL CONFORM TO 300.6 REINFORCEMENT 120.2 SHOP DRAWINGS TO BE SUBMITTED SHALL PROVIDE COMPLETE GENERAL NOTES INDICATED OR NOTED ON DRAWINGS: INFORMATION FOR THE PRODUCTS OR COMPONENTS TO BE ASTM A510 OR ASTM A108 STEEL AND THE NUT SHALL CONFORM SUPPLIED. SUBMITTAL INFORMATION SHALL INCLUDE. BUT NOT BE A. DEFORMED BARS TO ASTM A563, GRADE A. ASTM A615, A. SNUG-TIGHTENED JOINTS: ALL SIMPLE SHEAR CONNECTIONS. LIMITED TO: MEMBER SIZES AND DIMENSIONS; GRADES OF MATERIAL GRADE 60 100. DESIGN CRITERIA 350.8 ALL EPOXY ADHESIVE ANCHORS FOR ANCHORING TO HOLLOW B. SLIP-CRITICAL JOINTS: ALL LATERAL BRACING AND MOMENT FURNISHED; MATERIAL PREPARATION REQUIRED; MATERIAL FINISH AND B. WELDED WIRE FABRIC_____ MASONRY SHALL BE "HILTI HIT HY20 ADHESIVE ANCHORS" AS MATERIAL COATINGS TO BE FURNISHED; INFORMATION REGARDING ASTM A185 100.1 DESIGN BUILDING CODE: C. PRETENSIONED JOINTS: CONNECTIONS WHERE A490 BOLTS ARE CUTS, COPES AND HOLES REQUIRED FOR OTHER TRADES; END MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. (OR EQUAL). FURNISHED AND ARE IN TENSION OR COMBINED SHEAR AND 300.7 MINIMUM COVER FOR CAST-IN-PLACE CONCRETE REINF., UNLESS CONNECTIONS; CAMBER AND OTHER DEVIATION FROM LINE; SPECIAL A. INTERNATIONAL BUILDING CODE, 2006 OTHERWISE SHOWN ON DRAWINGS, SHALL BE AS FOLLOWS: ERECTION AND/OR INSTALLATION PROCEDURES INCLUDING 350.9 ALL EXPANSION ANCHORS FOR ANCHORING TO MASONRY BE "HILTI 100.2 GRAVITY LOADS: REQUIREMENTS FOR TEMPORARY STABILIZATION. HLC SLEEVE ANCHORS" AS MANUFACTURED BY HILTI FASTENING 510.6 THE USE OF TENSION-CONTROL (T.C.) BOLTS IN SNUG-TIGHTENED A. FOOTINGS & GRADE BEAMS____ SYSTEMS, INC. (OR EQUAL). JOINTS IS PROHIBITED. 120.3 THE CONTRACTOR SHALL NOT DIRECTLY INCORPORATE THE A. FLOOR LIVE LOADS: B. FOUNDATION WALLS____ STRUCTURAL DRAWINGS OR PORTIONS THEREOF INTO SHOP 350.10 THE SPACING, MINIMUM EMBEDMENT, AND INSTALLATION OF THE 510.7 ALL BOLTED SLIP-CRITICAL JOINTS SHALL HAVE FAYING SURFACES 1. CLINICAL AND SUPPORT SERVICES DRAWINGS OR ERECTION DRAWINGS TO BE SUBMITTED FOR THIS ANCHORS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S PREPARED AS REQUIRED TO FURNISH CLASS A SLIP RESISTANCE IN PROJECT WITHOUT FIRST OBTAINING THE EXPRESS WRITTEN 2000 LB. C. COLUMNS & PEDESTALS (OVER VERT. REINF.)___ RECOMMENDED PROCEDURES. ACCORDANCE WITH THE APPLICABLE PROVISIONS OF THE RCSC PERMISSION OF ATLANTIC ENGINEERING SERVICES. SUBMITTED SHOP "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR 2. CORRIDORS 100 PSF DRAWINGS WHICH CONTAIN COPIES OR REPRODUCTIONS OF ANY D. SLABS CAST AGAINST EARTH PORTION OF THE STRUCTURAL DRAWINGS WITHOUT THE EXPRESS A490 BOLTS" (LATEST EDITION). SLABS, DEPTH/3 TOP COVER FOR SLABS GREATER THAN 4" 420. MASONRY 150 PSF WRITTEN PERMISSION OF ATLANTIC ENGINEERING SERVICES WILL BE RETURNED REJECTED. PERMISSION FOR A SPECIFIC CONTRACTOR OR 510.8 THE CHECKING OF DESIGN SLIP RESISTANCE FOR SLIP-CRITICAL 300.8 SPLICES IN REINFORCEMENT, WHERE PERMITTED, SHALL BE AS CONNECTIONS SHALL BE AT THE SERVICE-LOAD LEVEL. 4. STAIRS SUB-CONTRACTOR TO USE PORTIONS OF THE STRUCTURAL FOLLOWS: 420.1 ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE LATEST DRAWINGS IN THEIR PREPARATION OF SHOP DRAWINGS REQUIRES EDITION OF "BUILDING CODE REQUIREMENTS FOR MASONRY 510.9 ALL WELDING SHALL BE IN ACCORDANCE WITH THE STRUCTURAL B. ROOF LIVE LOADS: THAT CONTRACTOR OR SUB-CONTRACTOR TO ENTER INTO A WRITTEN STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY A. WELDED WIRE MESH___ WELDING CODE, AWS D1.1, LATEST EDITION. OF THE AMERICAN AGREEMENT WITH ATLANTIC ENGINEERING SERVICES AND TO PAY A STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE. SERVICE FEE. SUCH AGREEMENT IS NON-TRANSFERRABLE AND IS WELDING SOCIETY. ELECTRODES SHALL BE E70XX FOR MANUAL ARC FLAT ROOF B. ALL OTHERS_____ CLASS "B" TENSION. EXTENDED ONLY TO THAT CONTRACTOR FOR THE DURATION OF THIS WELDING AND F7X-EXXX FOR SUBMERGED ARC WELDING. CASE "1" MINIMUM, 420.2 ALL MASONRY WORK TO BE EXECUTED IN COLD WEATHER SHALL BE 2. PITCHED ROOF UNLESS OTHERWISE IN CONFORMANCE WITH THE RECOMMENDATIONS FOR COLD WEATHER 510.10 ALL BEAM-TO-COLUMN CONNECTIONS SHALL BE DESIGNED FOR THE NOTED 120.4 THE CONTRACTOR SHALL SUBMIT PRINTED COPIES OF SHOP CONSTRUCTION OF THE LATEST EDITION OF "BUILDING CODE 100.3 LATERAL LOADS: SUMMATION OF THE FOLLOWING LOADS: DRAWINGS FOR REVIEW BY ATLANTIC ENGINEERING SERVICES REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530) AND THE 300.9 CLASS "B", CASE "1" TENSION SPLICES IN INCHES, SHALL BE AS A. FOR ONE HALF THE UNIFORM LOAD CAPACITY OF THE MEMBER A. WIND LOADS (IN ACCORDANCE WITH DESIGN BUILDING CODE PER PRINTED COPIES OF SHOP DRAWINGS SHALL CONSIST OF FOUR (4) "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE IN ACCORDANCE WITH AISC SPECIFICATIONS, BUT NOT LESS REPRODUCIBLE PRINTS. UPON THE COMPLETION OF THE SHOP AMERICAN CONCRETE INSTITUTE WITH THE FOLLOWING ADDITION TO GENERAL NOTE 100.1): THE REQUIREMENTS OF ACI 530.1 SECTION 1.8-B: FOR ALL DRAWING REVIEW, THREE (3) PRINTS SHALL BE RETURNED TO THE 3000 PSI CONDITIONS WHEN TEMPERATURES FALL BELOW 40 DEGREES F, THE PROJECT ARCHITECT. ONE (1) SHALL BE RETAINED BY THE 1. BASIC WIND SPEED (3 SECOND GUST) = 120 MPH TOP BARS ALL OTHERS TOP BARS ALL OTHERS B. A 10.0 KIP AXIAL FORCE (ACTING IN TENSION AND TEMPERATURE OF THE NEWLY LAID MASONRY OR NEWLY GROUTED ARCHITECT, ONE (1) SHALL BE DISTRIBUTED TO THE OWNER. AND #3 (#10) 28 MASONRY SHALL BE MAINTAINED ABOVE 32 DEGREES (F) FOR A COMPRESSION). 2. BUILDING OCCUPANCY CATEGORY = II ONE (1) SHALL BE SENT TO THE CONTRACTOR FOR DISTRIBUTION #4 (#13) MINIMUM OF 24 HOURS USING THE METHODS DESCRIBED IN ACI AS REQUIRED. C. THE EFFECTS OF CONCENTRATED LOADS OCCURRING CLOSE TO **#**5 (**#**16) 47 40 530.1. 3. WIND LOAD IMPORTANCE FACTOR (Iw) = 1.0THE ENDS OF THE BEAMS. #6 (#19) 48 120.5 SUBJECT TO THE APPROVAL OF ATLANTIC ENGINEERING SERVICES. 420.3 MORTAR SHALL CONFORM TO THE PROPORTION SPECIFICATION OF 4. EXPOSURE CATEGORY = B (#22) THE CONTRACTOR MAY SUBMIT ELECTRONIC COPIES OF SHOP ASTM C270, TYPE M OR S. PROVIDE TYPE M MORTAR AT ALL HIGH 510.11 ALL NON-COMPOSITE BEAM END CONNECTIONS SHALL BE DESIGNED, DRAWINGS IN LIEU OF PRINTED COPIES. ELECTRONIC COPIES SHALL #8 (#25) UNLESS NOTED OTHERWISE, FOR AN END REACTION "R" EQUAL TO STRENGTH MASONRY NOTED AS F'm = 2500 PSI OR GREATER. 5. ENCLOSURE CLASSIFICATION = ENCLOSED BE SUBMITTED TO ATLANTIC ENGINEERING SERVICES IN ADOBE 105 #9 (#29) PROVIDE TYPE S MORTAR AT ALL STRUCTURAL MASONRY AND NOT LESS THAN ONE HALF THE UNIFORM LOAD CAPACITY OF THE ACROBAT FILE FORMAT, WITH ONE (1) ELECTRONIC FILE PER 102 #10 (#32) 118 REINFORCED MASONRY UNLESS NOTED OTHERWISE. MEMBER IN ACCORDANCE WITH AISC SPECIFICATIONS, BUT NOT LESS 6. INTERNAL PRESSURE COEFFICIENT (GCpi) = "0.18 PACKAGE SUBMISSION. ATLANTIC ENGINEERING SERVICES WILL PRINT, 131 #11 (#36) THAN 6 KIPS. THE EFFECTS OF CONCENTRATED LOADS OCCURRING REVIEW, RE-SCAN THE REVIEWED SHOP DRAWINGS WITH ALL CLOSE TO THE ENDS OF THE BEAMS SHALL BE CONSIDERED IN THE 420.4 GROUT SHALL CONFORM TO ASTM C476 AND AS FOLLOWS: 7. COMPONENTS AND CLADDING PRESSURES: SEE COMMENTS/NOTATIONS, AND RETURN ONE (1) ELECTRONIC PACKAGE 300.10 SPLICES IN TOP REINFORCEMENT SHALL BE MADE AT MIDSPAN. CONNECTION DESIGN. "COMPONENTS AND CLADDING WIND LOADS" TABLE, AND TO THE ARCHITECT TO PRINT, REVIEW, RE-SCAN, AND DISTRIBUTE TO SPLICES IN BOTTOM REINFORCEMENT SHALL BE OVER SUPPORTS A. COMPRESSIVE STRENGTH (F'c) OF GROUT = F'm AS INDICATED "COMPONENTS AND CLADDING WIND PRESSURE DIAGRAM" THE CONTRACTOR. UNLESS NOTED OTHERWISE 510.12 ALL COMPOSITE BEAM END CONNECTIONS SHALL BE DESIGNED, BELOW BUT NO LESS THAN 2,000 PSI. UNLESS NOTED OTHERWISE, FOR A REACTION "Rc" EQUAL TO NOT 120.6 THE REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS FOR THIS 300.11 TOP BARS IN BEAMS SHALL TERMINATE IN A CLASS "B" TENSION B. SLUMP OF GROUT SHALL BE 8 TO 11 INCHES AS MEASURED LESS THAN THE FOLLOWING: 110. GENERAL PROJECT IS FOR CONFORMANCE WITH THE DESIGN CONCEPT AND SPLICE OR HOOK AT DISCONTINUOUS END. ACCORDING TO ASTM C143. FOR GENERAL COMPLIANCE WITH THE INFORMATION CONTAINED IN A. FOR BEAM DEPTHS GREATER THE CONTRACT DOCUMENTS. COMMENTS REGARDING THESE 300.12 PARALLEL REINFORCEMENT PLACED IN TWO OR MORE LAYERS SHALL C. MAX. AGGREGATE SIZE SHALL BE 3/8" (AGGREGATE GRADED TO $Rc = 1.50 \times R$ THAN 21"_ 110.1 THIS DRAWING HAS BEEN PRODUCED ENTIRELY ON ATLANTIC SUBMITTALS DO NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE HAVE A CLEAR DISTANCE BETWEEN LAYERS OF 1". UPPER LAYER PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C476 AND WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE ENGINEERING SERVICES CADD SYSTEM. ANY OTHER LETTERING, LINES BARS SHALL BE PLACED DIRECTLY ABOVE BARS IN THE BOTTOM B. FOR BEAM DEPTHS GREATER FOR PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER. OR SYMBOLS, OTHER THAN PROFESSIONAL STAMPS AND SIGNATURES, THAN OR EQUAL TO 14", HAVE BEEN MADE WITHOUT THE AUTHORIZATION OF ATLANTIC BUT LESS THAN OR EQUAL TO 21" $_$ Rc = 2.00 x R 420.5 LIMIT CEMENTITIOUS MATERIALS IN MORTAR TO: PORTLAND CEMENT ENGINEERING SERVICES AND ARE INVALID. 300.13 ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH CONFORMING TO ASTM C150 TYPE I; LIME CONFORMING TO ASTM 200. FOUNDATIONS — GENERAL STANDARD ACCESSORIES DURING PLACEMENT OF CONCRETE. C207; MORTAR CEMENT CONFORMING TO ASTM C1329 AND MASONRY C. FOR BEAM DEPTHS GREATER THAN 110.2 THE STRUCTURAL DRAWINGS SHALL GOVERN THE WORK FOR ALL REINFORCING SUPPORTS FOR ALL EXPOSED CONCRETE SHALL BE CEMENT CONFORMING TO ASTM C91. STRUCTURAL FEATURES, UNLESS NOTED OTHERWISE. THE OR EQUAL TO 8", GALVANIZED WITH PLASTIC COATED FEET (ALL WELDED WIRE MESH ARCHITECTURAL DRAWINGS SHALL GOVERN THE WORK FOR ALL 200.1 NO BACKFILLING AGAINST FOUNDATION WALLS SHALL BE PERMITTED BUT LESS THAN 14" ____ $Rc = 2.25 \times R$ SHALL BE CHAIRED IN ACCORDANCE WITH THE 2001 FBC). 420.6 PROVIDE SOLID AND HOLLOW LOAD BEARING CONCRETE BLOCK UNITS UNTIL SUPPORTING STRUCTURAL ELEMENTS HAVE BEEN PLACED AND DIMENSIONS. HAVE BECOME CAPABLE OF FURNISHING THE NECESSARY SUPPORT PER ASTM C90, TYPE N-II, AS REQUIRED TO PROVIDE F'm AS WHERE THE REACTION "R" IS EQUAL TO ONE HALF THE UNIFORM 300.14 ALL TIES SHALL HAVE 135 DEGREE HOOKS. 110.3 DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONS. ONLY DIMENSIONS FOR THE WALLS. PROVIDE TEMPORARY SHORING WHERE REQUIRED. NOTED BELOW. LOAD CAPACITY OF THE MEMBER IN ACCORDANCE WITH AISC WHERE BACKFILL IS REQUIRED ON BOTH SIDES OF THE WALL INDICATED ON DRAWINGS MAY BE USED TO ESTABLISH THE LOCATION SPECIFICATIONS, BUT NOT LESS THAN 6 KIPS. THE EFFECTS OF 300.15 PROVIDE 1/2" PREMOULDED EXPANSION MATERIAL WHERE SLAB ON BACKFILL BOTH SIDES SIMULTANEOUSLY WITH A GRADE DIFFERENCE 420.7 MINIMUM 28-DAY ULTIMATE COMPRESSIVE STRENGTH OF MASONRY: AND EXTENT OF STRUCTURAL WORK. IF A REQUIRED DIMENSION IS CONCENTRATED LOADS OCCURRING CLOSE TO THE ENDS OF THE GRADE IS POURED AROUND COLUMNS AND AGAINST WALLS UNLESS NOT FURNISHED ON DRAWINGS, THE CONTRACTOR SHALL SUBMIT A NOT TO EXCEED 2'-0" AT ANY TIME. CONTRACTOR SHALL USE BEAMS SHALL BE CONSIDERED IN THE CONNECTION DESIGN. OTHERWISE SHOWN ON DRAWINGS. REQUEST FOR INFORMATION TO OBTAIN THE DIMENSION EXTREME CAUTION DURING BACKFILLING TO PREVENT DAMAGE TO FOUNDATION WALLS. THE USE OF HEAVY EQUIPMENT FOR 510.13 ALL HEADED SHEAR STUDS SHALL CONFORM TO ASTM A 108, GRADE 300.16 CONTRACTION JOINTS FOR SLABS ON GRADE SHALL BE SPACED NO 110.4 UNLESS OTHERWISE INDICATED, PROVIDE EQUAL SPACING OF 420.8 HORIZONTAL JOINT REINFORCING FOR ALL EXTERIOR AND LOAD BACKFILLING IS NOT RECOMMENDED. 1015 OR 1020, COLD FINISHED CARBON STEEL. MORE THAN 16'-0" ON CENTER. PANELS FORMED BY JOINTS OR STRUCTURAL COMPONENTS BETWEEN OVERALL DIMENSIONS INDICATED BEARING WALLS SHALL BE GALVANIZED TRUSS OR LADDER TYPE SLAB EDGES SHALL BE AS SQUARE AS POSSIBLE WITH A 200.2 THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE DUR-O-WAL OR EQUIVALENT AS APPROVED BY THE ENGINEER WITH 510.14 BEAMS MARKED [N] ARE COMPOSITE AND ARE TO HAVE "N" 3/4" LENGTH-TO-WIDTH RATIO NOT TO EXCEED 1.5. AND TAKE THE NECESSARY PRECAUTIONS TO INSURE THAT THE 2-9 GAGE LONGITUDINAL WIRE AND 9 GAGE CROSS WIRE, SPACED DIAMETER BY 4" TALL, HEADED SHEAR STUDS WELDED TO THE TOP 110.5 THE METHOD AND FREQUENCY OF ATTACHING MECHANICAL FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION. AT 16" CENTER TO CENTER, UNLESS NOTED OTHERWISE. PROVIDE 300.17 CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL OF THE BEAM SPACED ACCORDING TO THE NOTES AND TYPICAL EQUIPMENT UNITS, ETC., TO THE STRUCTURAL ELEMENTS SHALL BE PROVIDE FOR DEWATERING AS NECESSARY. ADDITIONAL LAYERS OF JOINT REINFORCEMENT IN THE FIRST TWO SLOTS, PIPE SLEEVES, ETC., AS REQUIRED FOR MECHANICAL TRADES SUBJECT TO THE ENGINEERS REVIEW AND APPROVAL COURSES ABOVE AND BELOW A MASONRY OPENING. PROVIDE LAP AS BEFORE CONCRETE IS PLACED. 200.3 THE CONTRACTOR SHALL USE EXTREME CAUTION DURING EXCAVATION. RECOMMENDED BY MANUFACTURER WITH A MINIMUM OF 6". 510.15 WHERE THE DECK IS PERPENDICULAR TO SUPPORTING COMPOSITE 110.6 UNLESS OTHERWISE INDICATED, STRUCTURAL COMPONENTS SUCH EXCAVATION SHALL BE PERFORMED IN SUCH A MANNER AS DISCONTINUE JOINT REINFORCING AT CONTROL JOINTS. PROVIDE "L" BEAMS, SHEAR STUDS SHALL BE PLACED AS FOLLOWS, UNLESS 300.18 CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL SUPPORTING MECHANICAL EQUIPMENT HAVE NOT BEEN DESIGNED TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING SHAPE AND "T" SHAPE DUR-O-WAL AT ALL INTERSECTION CORNERS NOTED OTHERWISE: SLOTS, PIPE SLEEVES, ETC., AS REQUIRED FOR MECHANICAL TRADES FOR THE VIBRATIONAL EFFECTS OF THE EQUIPMENT. THE STRUCTURES TO REMAIN. PROVIDE TEMPORARY SHORING AS BEFORE CONCRETE IS PLACED. CONTRACTOR SHALL SUBMIT SHOP WITH 8" MINIMUM LAP. SEE TYPICAL DETAILS. CONTRACTOR SHALL PROVIDE VIBRATION ISOLATORS FOR ANY REQUIRED. 510.16 WHERE THE NUMBER OF DECK CELLS AVAILABLE IS GREATER THAN DRAWINGS SHOWING LOCATIONS FOR ALL SLAB OPENINGS FOR MECHANICAL EQUIPMENT MOUNTED TO THE STRUCTURE IN THE NUMBER OF STUDS, THE STUDS SHALL BE SPACED EQUALLY 420.9 FULL BED AND HEAD JOINTS SHALL BE USED. REVIEW BY STRUCTURAL ENGINEER. ACCORDANCE WITH THE EQUIPMENT MANUFACTURER'S 200.4 CONCRETE SLABS ON GRADE HAVE BEEN DESIGNED TO BEAR ON ALONG THE LENGTH OF BEAM. COMPACTED SUBGRADE SOILS OR PROPERLY COMPACTED FILL. RECOMMENDATIONS. 420.10 ALL MASONRY WALLS SHALL BE SECURELY BRACED UNTIL FLOOR OR 300.19 PIPES OR CONDUITS PLACED IN SLABS SHALL NOT HAVE AN 510.17 WHERE THE NUMBER OF DECK CELLS AVAILABLE IS LESS THAN THE ROOF SYSTEM HAS BEEN INSTALLED AND HAS BECOME CAPABLE OF OUTSIDE DIAMETER LARGER THAN 1/3 THE SLAB THICKNESS AND 110.7 THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, NUMBER OF STUDS, THE STUDS SHALL BE SPACED ONE PER DECK STABILIZING THE WALLS. SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTERS DIMENSIONS, ETC., AND SHALL NOTIFY THE ARCHITECT OF ANY AND 210. SHALLOW FOUNDATIONS CELL ALONG THE LENGTH OF THE BEAM STARTING AT EACH END. ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE. NO ALL DISCREPANCIES, ADDITIONAL INFORMATION, ETC., BEFORE 420.11 GROUT SOLID ALL CELLS IN MASONRY UNITS INSTALLED BELOW THE REMAINING STUDS SHALL BE PLACED IN A TWO/CELL CONDUITS SHALL BE PLACED IN SLAB WITHIN 12" OF COLUMN FACE BEGINNING THE WORK. CONFIGURATION AT EACH END OF THE BEAM AS PER THE TYPICAL 210.1 FOUNDATIONS HAVE BEEN DESIGNED FOR A MINIMUM ALLOWABLE OR FACE OF BEARING WALL. NO CONDUITS MAY BE PLACED IN EXTERIOR SLABS. 110.8 THE CONTRACTOR SHALL USE EXTREME CAUTION IN THE DEMOLITION BEARING PRESSURE OF 1500 PSF WHICH SHALL BE VERIFIED BY A 420.12 GROUT SOLID ALL CELLS CONTAINING REINFORCING, AND WHERE GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. OF EXISTING STRUCTURES. SUCH DEMOLITION SHALL BE PERFORMED 510.18 WHERE THE DECK IS PARALLEL TO SUPPORTING COMPOSITE BEAMS, INDICATED ON PLANS AND SECTIONS. 300.20 PRIOR TO CONCRETE PLACEMENT, THE CONTRACTOR SHALL SUBMIT A IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF STUDS SHALL BE EQUALLY SPACED OVER THE LENGTH OF THE 210.2 SPREAD FOOTINGS HAVE BEEN DESIGNED TO BEAR ON UNDISTURBED CONCRETE MIX DESIGN PREPARED IN ACCORDANCE WITH THE ALL EXISTING STRUCTURES TO REMAIN. PROVIDE SHORING AS 420.13 PROVIDE FINE GROUT PER ASTM C476 WHEN WIDTH OF GROUT MEMBERS UNLESS NOTED OTHERWISE AS PER THE TYPICAL DETAILS. SPECIFICATIONS TO THE STRUCTURAL ENGINEER FOR REVIEW. SOILS OR PROPERLY COMPACTED FILL HAVING AN ALLOWABLE SPACE IS LESS THAN 2". PROVIDE COARSE GROUT FOR GROUT BEARING CAPACITY OF 3000 PSF. 510.19 WHERE SHEAR STUDS ARE NOT INDICATED, BUT WHERE A NON SPACE WIDTHS 2" OR GREATER. PROVIDE FINE GROUT WHEN 300.21 ALL FIBER REINFORCED CONCRETE SHALL CONTAIN 100 % VIRGIN 110.9 THE CONTRACTOR SHALL PREPARE A WRITTEN DEMOLITION PLAN TO COMPOSITE BEAM SUPPORTS A CONCRETE SLAB, PROVIDE SHEAR 210.3 ELEVATIONS SHOWN ON THE DRAWINGS AT WHICH FOUNDATIONS ARE POLYPROPYLENE, COLLATED, FIBRILLATED FIBERS SPECIFICALLY REINFORCING HAS LESS THAN 1/2" CLEARANCE. BE SUBMITTED TO THE ARCHITECT FOR REVIEW. THIS PLAN IS TO STUDS AT A MAXIMUM SPACING OF 24" CENTER TO CENTER. MANUFACTURED FOR USE AS A SECONDARY CONCRETE INDICATE, AS A MINIMUM, SEQUENCE OF DEMOLITION OPERATIONS, TO BEAR ARE APPROXIMATE. MATERIAL ON WHICH FOUNDATIONS ARE TO BEAR SHALL HAVE AT LEAST THE ABOVE NOTED CAPACITY. REINFORCEMENT. VOLUME PER CUBIC YARD OF CONCRETE SHALL BE 420.14 PROVIDE CLEAN OUT AND INSPECTION HOLES AT BOTTOM OF LOCATION OF PROPOSED TEMPORARY SHORING, SCAFFOLDING, 510.20 CUTS, HOLES AND COPING, ETC. REQUIRED FOR OTHER TRADES MASONRY WALL IN ACCORDANCE WITH MASONRY CODE AT A MINIMUM OF 0.1 PERCENT (1 1/2 LBS. PER CU YD.). FIBER ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF ONE FOOT BELOW BRACING, ETC., AND PROPOSED METHOD OF DEMOLITION. WHERE SHALL BE SHOWN ON THE SHOP DRAWING AND MADE IN THE SHOP. MANUFACTURER MUST PROVIDE DOCUMENTATION INDICATING A REQUIRED. THE CONTRACTOR SHALL RETAIN THE SERVICES OF A FINISHED GRADE. REINFORCING IF HIGH LIFT GROUTING (OVER 4 FEET HIGH) IS USED. CUTS OR BURNING OR HOLES IN STRUCTURAL STEEL IN THE FIELD REGISTERED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE MINIMUM OF FIVE YEARS SATISFACTORY PERFORMANCE. FIBERS WILL NOT BE PERMITTED. SHALL BE 3/4" TO 1-1/2" IN LENGTH, WITH A MINIMUM TENSILE 420.15 DEFORMED BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, STATE OF FLORIDA FOR THE DESIGN OF TEMPORARY SHORING AND 250.1 THE OWNER/CONTRACTOR SHALL RETAIN THE SERVICES OF A BRACING. THE REVIEW OF THE PROPOSED DEMOLITION PLAN IS FOR GRADE 60. PROVIDE LAP SPLICES PER THE TABLE BELOW. PROVIDE PROFESSIONAL GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL STRENGTH OF 80 KSI. 510.21 ALTERNATE CONNECTION DETAILS MAY BE USED IF SUCH DETAILS CONFORMANCE WITH THE DESIGN CONCEPT AND FOR GENERAL BAR SPACERS AS REQUIRED TO PROPERLY LOCATE REINFORCING. OF THE ARCHITECT, TO INSPECT THE FOUNDATIONS, BEARING LEVELS ARE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT ETC.. AND VERIFY THAT THE MATERIAL ON WHICH FOUNDATIONS BEAR 340.1 THE CONTRACTOR SHALL PROVIDE FOR AN AVERAGE OF 5/8" OF HOWEVER, THE ENGINEER SHALL BE THE SOLE JUDGE OF DOCUMENTS. COMMENTS REGARDING THESE SUBMITTALS DO NOT HAS AT LEAST THE ABOVE NOTED CAPACITY AS PER 210.2. ADDITIONAL CONCRETE THICKNESS DURING PLACEMENT OF ALL SLABS ACCEPTANCE AND THE CONTRACTOR'S BID SHALL ANTICIPATE THE RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE CONTRACT SUPPORTED AND FORMED ON STEEL DECK OVER THE ENTIRE FLOOR USE OF THOSE SPECIFIED DETAILS SHOWN ON THE DRAWINGS THE DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING AREA. THE DESIGN OF THE SUPPORTING FRAMING INCLUDES THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF SUCH ALTERNATE HIS WORK IN A SAFE AND SATISFACTORY MANNER. 300. REINFORCED CONCRETE PROVISION FOR THE ADDITIONAL DEAD LOAD BASED ON THE SAME DETAILS WHICH HE PROPOSES. AVERAGE INCREASE IN THE SLAB THICKNESS RESULTING FROM THE 420.16 MASONRY COURSING SHOWN IN SECTION IS APPROXIMATE. REFER TO 110.10 ALL STRUCTURAL WORK SHALL BE INSPECTED IN ACCORDANCE WITH DEFLECTION OF THE UNSHORED FRAMING. THE CONTRACTOR SHALL PLANS AND ELEVATIONS FOR ACTUAL COURSING. COORDINATE ACTUAL 510.22 ALL STRUCTURAL STEEL FRAMES SHALL BE SECURELY BRACED UNTIL THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER 300.1 ALL REINFORCED CONCRETE WORK SHALL BE IN CONFORMANCE WITH PROVIDE THE MEANS BY WHICH THE MAXIMUM AND MINIMUM SLAB COURSING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. ALL FLOOR SLABS, ROOF DECKS AND SHEAR WALLS HAVE BEEN SHALL ENGAGE AN EXPERIENCED, QUALIFIED INSPECTION AGENCY, THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" THICKNESS CAN BE MONITORED AND VERIFIED DURING AND AFTER INSTALLED AND BECOME CAPABLE OF STABILIZING THE FRAMES. SUBJECT TO THE REVIEW OF THE ARCHITECT, TO PERFORM ALL THE PLACING AND FINISHING OPERATIONS. THE FINAL IN PLACE SLAB (ACI 318, LATEST EDITION) AND SPECIFICATIONS FOR STRUCTURAL INSPECTION WORK, AS REQUIRED. THICKNESS SHALL NOT BE LESS THAN THAT INDICATED ON THE CONCRETE (ACI 301, LATEST EDITION) OF THE AMERICAN CONCRETE 510. STRUCTURAL STEEL 510.23 ALL STRUCTURAL STEEL WORK, EXCEPT PORTIONS OF MEMBERS TO INSTITUTE. BE WELDED, FIELD BOLTED OR FIREPROOFED, SHALL BE SHOP 110.11 ALL PRE-FABRICATED STAIRS SHALL BE CAPABLE OF SUPPORTING PAINTED WITH PAINT CONFORMING TO STEEL STRUCTURES PAINTING THE SELFWEIGHT PLUS SUPERIMPOSED DEAD LOADS AND A 510.1 ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH 300.2 MINIMUM DESIGN COMPRESSION STRENGTH (F'c) REQUIRED AT 28 COUNCIL (SSPC) PAINT 25. APPLY PRIME PAINT ACCORDING TO SUPERIMPOSED LIVE LOAD OF 100 PSF AND THAT MEETS ALL THE 350. CONCRETE/MASONRY ANCHORS ANSI/AISC 360-05 - "SPECIFICATION FOR STRUCTURAL STEEL SSPC PAINT SYSTEM GUIDE NO. 7.00 CLEAN STEEL FREE OF LOOSE RELEVANT REQUIREMENTS OF THE LOCAL BUILDING CODE. THE BUILDINGS." LOADS, FORCES AND MOMENTS INDICATED ARE SERVICE SCALE, RUST, OIL AND GREASE. ADDITIONAL AREAS SHALL BE FIELD CONTRACTOR SHALL SUBMIT CALCULATIONS AND SHOP DRAWINGS A. FOUNDATIONS AND FOUNDATION WALLS LEVEL AND ARE INTENDED FOR USE WITH THE ALLOWABLE STRENGTH PAINTED AFTER WELDING. STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED TO 350.1 ALL HEADED CONCRETE ANCHORS SHALL BE NELSON 3/4" DIAMETER, DESIGN PROVISIONS OF THE CODE. PRACTICE IN THE STATE OF FLORIDA FOR REVIEW BY THE 4000 PSI 4" H4L ANCHORS WITH FLUXED ENDS AS MANUFACTURED BY B. SLABS ON GRADE__ 510.24 BEAMS AND GIRDERS SHALL BE CAMBERED AS SHOWN ON THE STRUCTURAL ENGINEER. NELSON STUD WELDING COMPANY, UNLESS THE SIZE IS NOTED 510.2 GRADE OF STEEL STRUCTURAL DRAWINGS. CAMBER INDICATED IS THE FINAL FIELD C. ELEVATED FLOOR SLABS_____ OTHERWISE ON THE STRUCTURAL DRAWINGS. CAMBER, INCLUDING ALL MILL TOLERANCES, AND SHOULD NOT BE 110.12 STEP FOOTINGS BELOW ALL SANITARY AND WATER LINES. PROVIDE A. STRUCTURAL >W= SHAPES_____ ASTM A992 STEPS IN THE FOUNDATIONS IN ACCORDANCE WITH THE TYPICAL 300.3 MAXIMUM WATER TO CEMENTITIOUS MATERIALS RATIO: 350.2 ALL HEADED CONCRETE ANCHORS SHALL BE MANUFACTURED FROM DETAILS. COORDINATE THE EXACT LOCATION AND ELEVATION OF THE MATERIAL WHICH CONFORMS TO ASTM A108, GRADES 1015 THROUGH B. STRUCTURAL >M=, >S=, 'C', 'MC> AND 'L= SHAPES_ASTM A36 510.25 FOR STEEL BEAMS WHERE NO INDUCED CAMBER IS INDICATED, PLUMBING LINES WITH THE MECHANICAL AND PLUMBING DRAWINGS A. FOUNDATIONS _____ 1020, HEADED-STUD TYPE, COLD-FINISHED CARBON STEEL; AWS FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS ORIENTED AND CONTRACTORS. PROVIDE SLEEVES IN THE FOUNDATION WALLS D1.1, TYPE B. UPWARD AFTER ERECTION. C. STEEL TUBES (HSS SHAPES)___ _ASTM A500, AS REQUIRED FOR PIPE PENETRATIONS. B. FOUNDATION WALLS___ GRADE B 350.3 ALL WELDS SHALL BE MADE IN ACCORDANCE WITH STRUCTURAL 510.26 BOLT AND WELD TESTING: C. SLABS ON GRADE_____ WELDING CODE ANSI/AWS D1.1-86 IF THE AMERICAN WELDING D. STEEL PIPE (ROUND HSS)_____ 120. SHOP DRAWINGS _ASTM A500, SOCIETY AND THE RECOMMENDATIONS OF THE NELSON STUD A. ALL SHOP AND FIELD BOLTS SHALL BE TESTED PER AISC GRADE B D. ELEVATED FLOOR SLABS_____ WELDING COMPANY. REQUIREMENTS. 120.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW BY E. PLATES AND BARS___ __ASTM A36 300.4 ALL CONCRETE FOR SLABS SHALL BE LIGHTWEIGHT CONCRETE (110 350.4 ALL ADHESIVE STUD ANCHORS SHALL BE "HILTI HIT-HY 150 B. ALL WELDS SHOULD BE VISUALLY INSPECTED. ATLANTIC ENGINEERING SERVICES AND THE PROJECT ARCHITECT. PCF MAXIMUM) WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE ADHESIVE CONCRETE ANCHORS" AS MANUFACTURED BY HILTI SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL 510.3 ALL BOLTED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS I, II OR I/II. MAXIMUM AGGREGATE SHALL BE 3/4" AND CONFORM C. TEN PERCENT OF ALL WELDS AT BEAM AND GIRDER SHEAR COMPONENTS INCLUDING, BUT NOT LIMITED TO THE FOLLOWING: FASTENING SYSTEMS, INC. (OR APPROVED EQUIVALENT). OF THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) TO ASTM C330. CONNECTIONS SHALL BE RANDOMLY INSPECTED BY MAGNETIC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR PARTICLE METHOD, COMPLYING WITH ASTM E109, PERFORMED ON A. FABRICATED STRUCTURAL STEEL 350.5 ALL EXPANSION STUD ANCHORS SHALL BE "HILTI KWIK-BOLT II A490 BOLTS" (LATEST EDITION). ROOT PASS AND ON FINISHED WELD. 300.5 ALL OTHER CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (144 EXPANSION CONCRETE ANCHORS" AS MANUFACTURED BY HILTI B. REINFORCING STEEL FOR CONCRETE AND MASONRY PCF MINIMUM) WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE FASTENING SYSTEMS, INC. (OR APPROVED EQUIVALENT). 510.4 ALL BOLTS SHALL BE ASTM A325, TYPE 1, 3/4" DIAMETER MINIMUM, D. ONE HUNDRED PERCENT OF FULL PENETRATION WELDS SHALL I, II OR I/II. MAXIMUM AGGREGATE SIZE SHALL BE 1-1/2" FOR UNLESS OTHERWISE NOTED. WHERE NECESSARY DUE TO CONNECTION FOOTINGS AND 3/4" FOR WALLS AND SLABS, CONFORMING TO ASTM 350.6 THE "HAS ANCHOR ROD" SHALL CONFORM TO ASTM A36 STEEL, THE REQUIREMENTS THE CONTRACTOR MAY UTILIZE ASTM A490, TYPE 1 "HAS SUPER ANCHOR ROD" SHALL CONFORM TO ASTM A193 STEEL, BOLTS. THE USE OF BOLTS WITH DIFFERENT ASTM DESIGNATIONS E. ONE HUNDRED PERCENT OF WELDS IN BEAM AND COLUMN D. SHEAR STUDS (NELSON STUDS) AND THE SAME DIAMETER IS PROHIBITED. AND THE NUT SHALL CONFORM TO ASTM A563, GRADE A. MOMENT CONNECTIONS SHALL HAVE ULTRASONIC INSPECTION, E. CONCRETE AND/OR MASONRY POST-INSTALLED ANCHORS F. OPEN WEB STEEL JOISTS 04-18-12 MEDICAL EQUIPMENT AND INTERIOR DESIGN REVISIONS G. LIGHT GAGE STEEL FRAMING Project Number REVISION DATE ARCHITECT/ENGINEERS: CONSULTANTS: Office of SPECIALTY TREATMENT EXPANSION 673-CSI-100 02/19/11 65% SUBMITTAL **GENERAL NOTES** 03/29/11 95% SUBMITTAL PROJECT- BUILDING #1 Construction **Building Number** 04/13/11 100% SUBMITTAL and Facilities 05/02/11 CONSTRUCTION SET Atlantic Engineering Services **Approved: Project Director** Drawing Number MEDICAL EQUIPMENT AND INTERIOR DESIGN 04/18/12 Management ttv architects, inc JAMES A. HALEY VETERANS' HOSPITAL acksonville 🗷 Florida 32211 hone: 904.743.4633 Fax: 904.725.9295 e elgntn JACKSONVILLE, FLORIDA 3220 l: jax@aespj.com Project No. 310180 SF001 Drawn FL COA #791 SC COA#1691 Department of Veterans Affairs www.ttvarch.com 02/19/11 PLC PAMELA LOWE COWDEN Dwg. 1 of 13 FL Lic#AA0002609 PE 51898

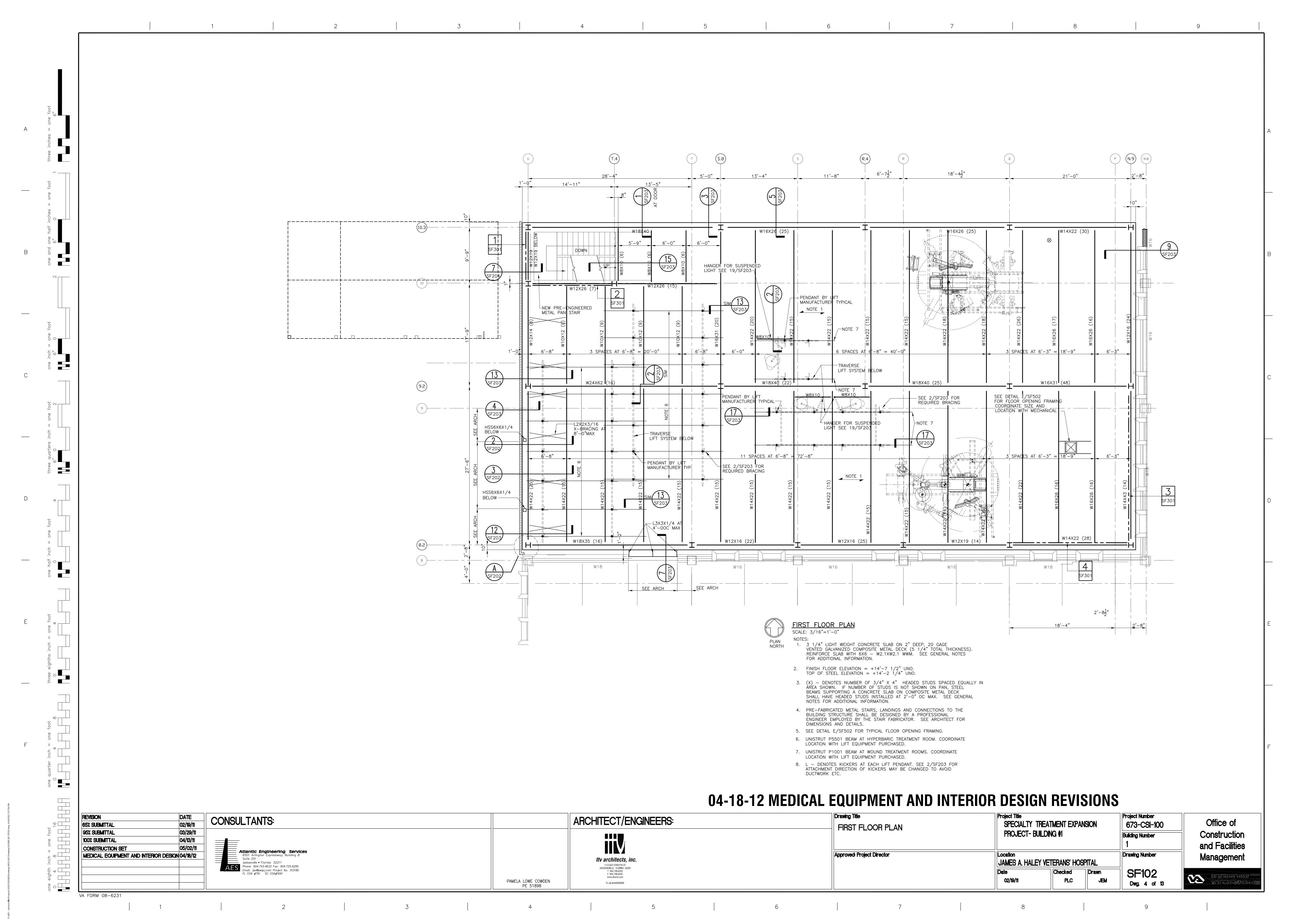
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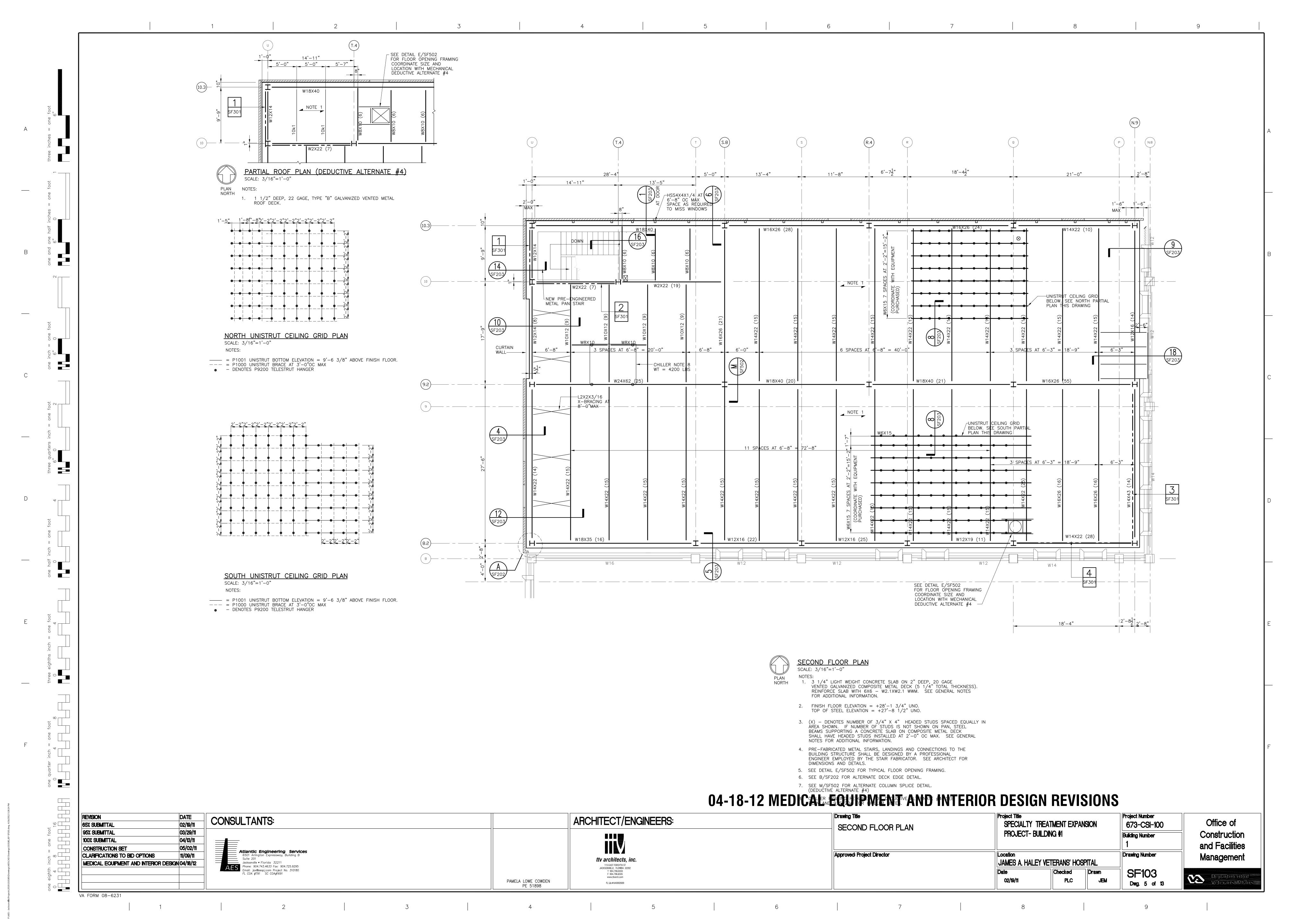


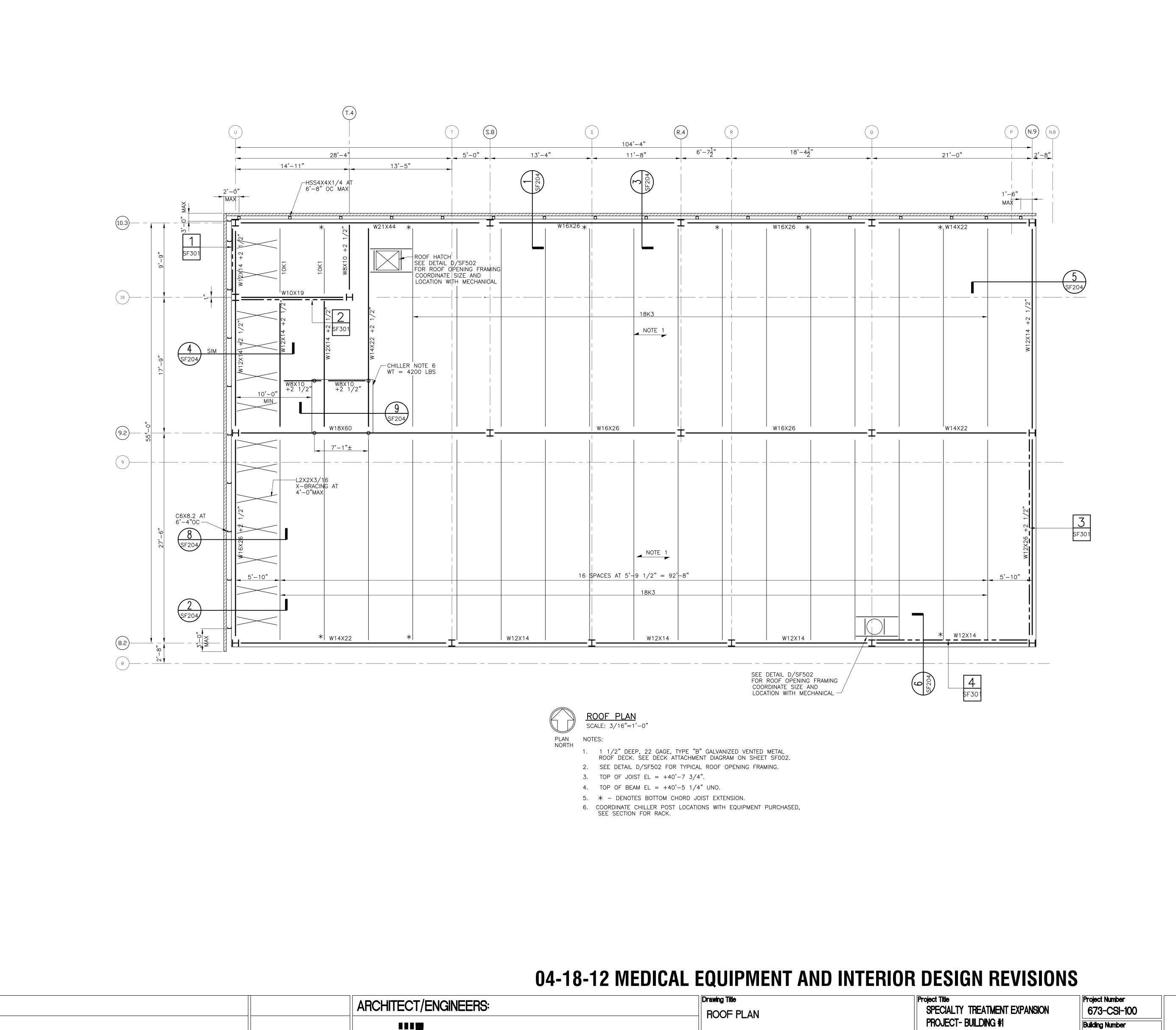


one eighth inch = one foot

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ttv architects, inc.

115 EAST FORSYTH ST JACKSONVILLE, FLORIDA 32202 T 904.798.8333 F 904.798.8335 www.ttvarch.com

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Department of Veterans Affairs

Drawing Number

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Dwg. 6 of 13

JAMES A. HALEY VETERANS' HOSPITAL

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02/19/11

one eighth inch = one foot

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REVISION

65% SUBMITTAL

95% SUBMITTAL

100% SUBMITTAL

VA FORM 08-6231

CONSTRUCTION SET

MEDICAL EQUIPMENT AND INTERIOR DESIGN 04/18/12

DATE 02/19/11

03/29/11

05/02/11

04/13/11

CONSULTANTS:

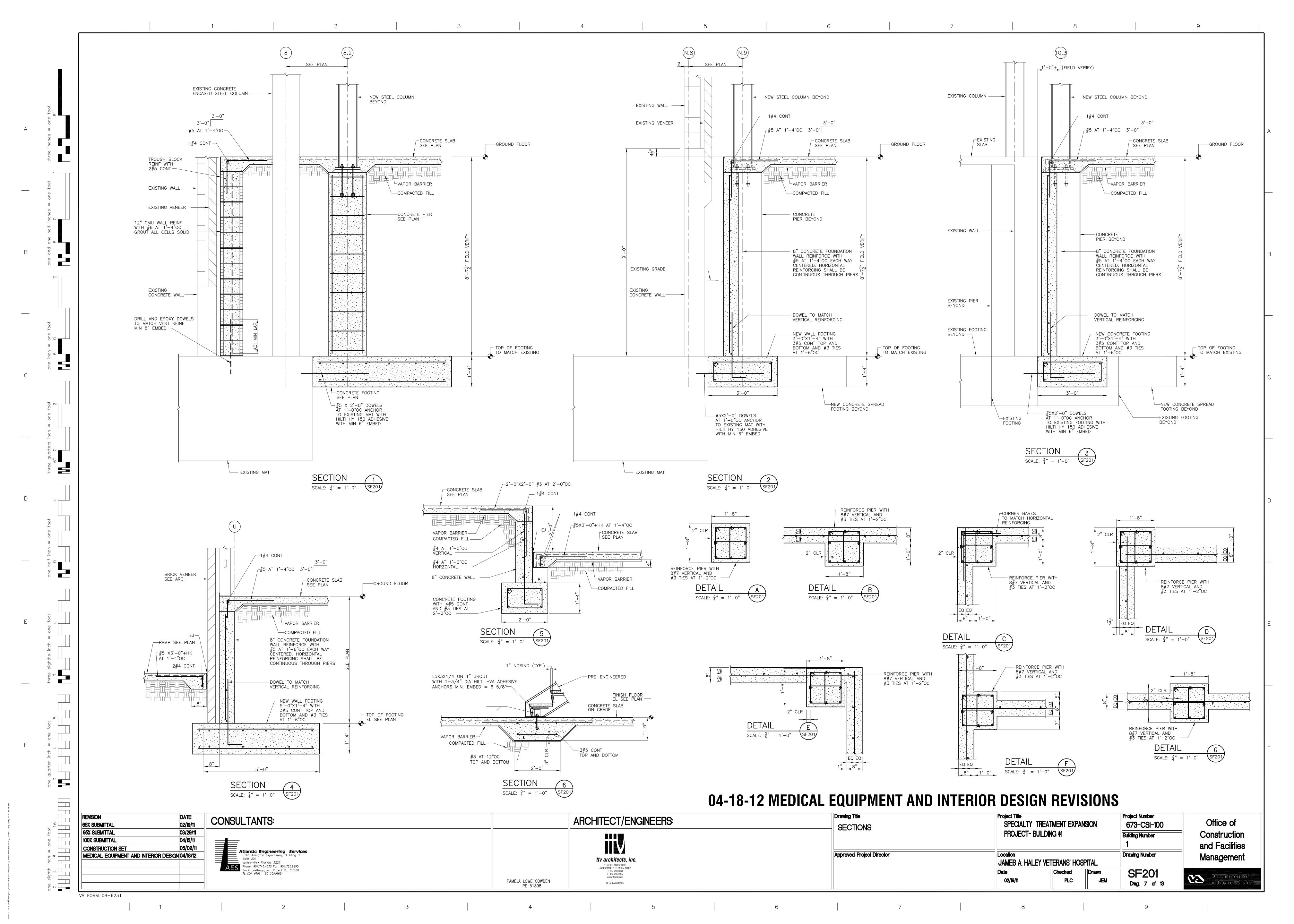
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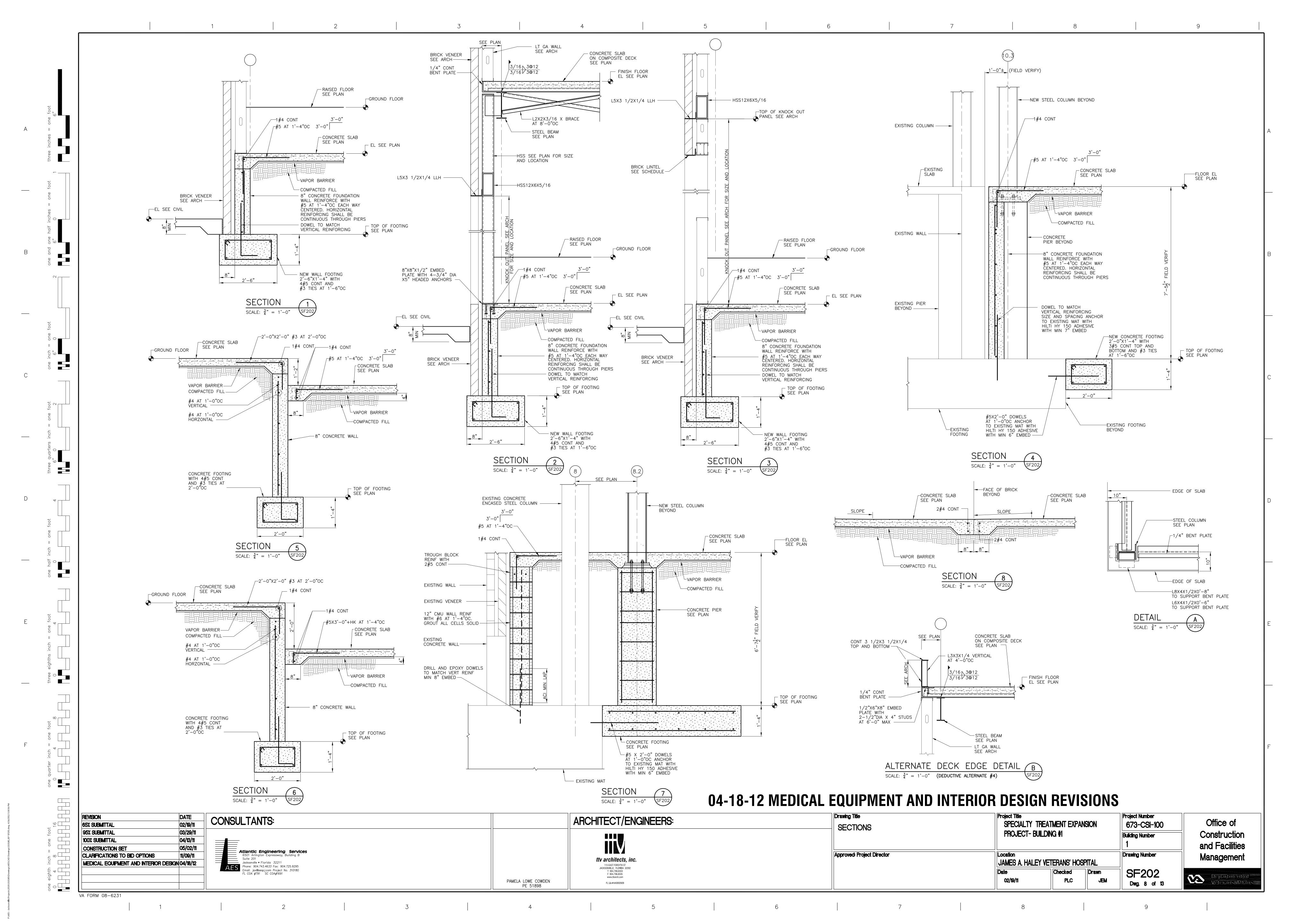
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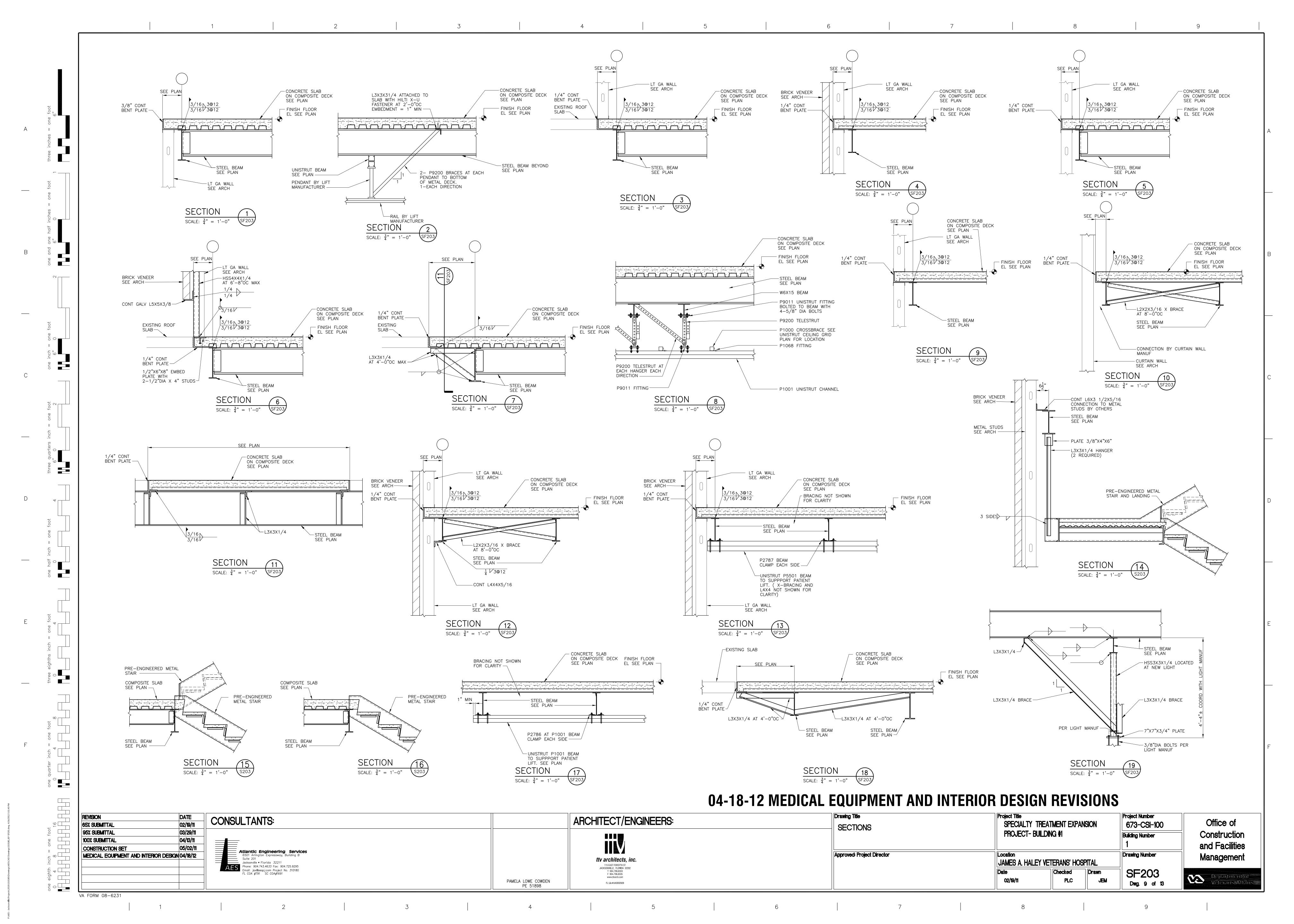
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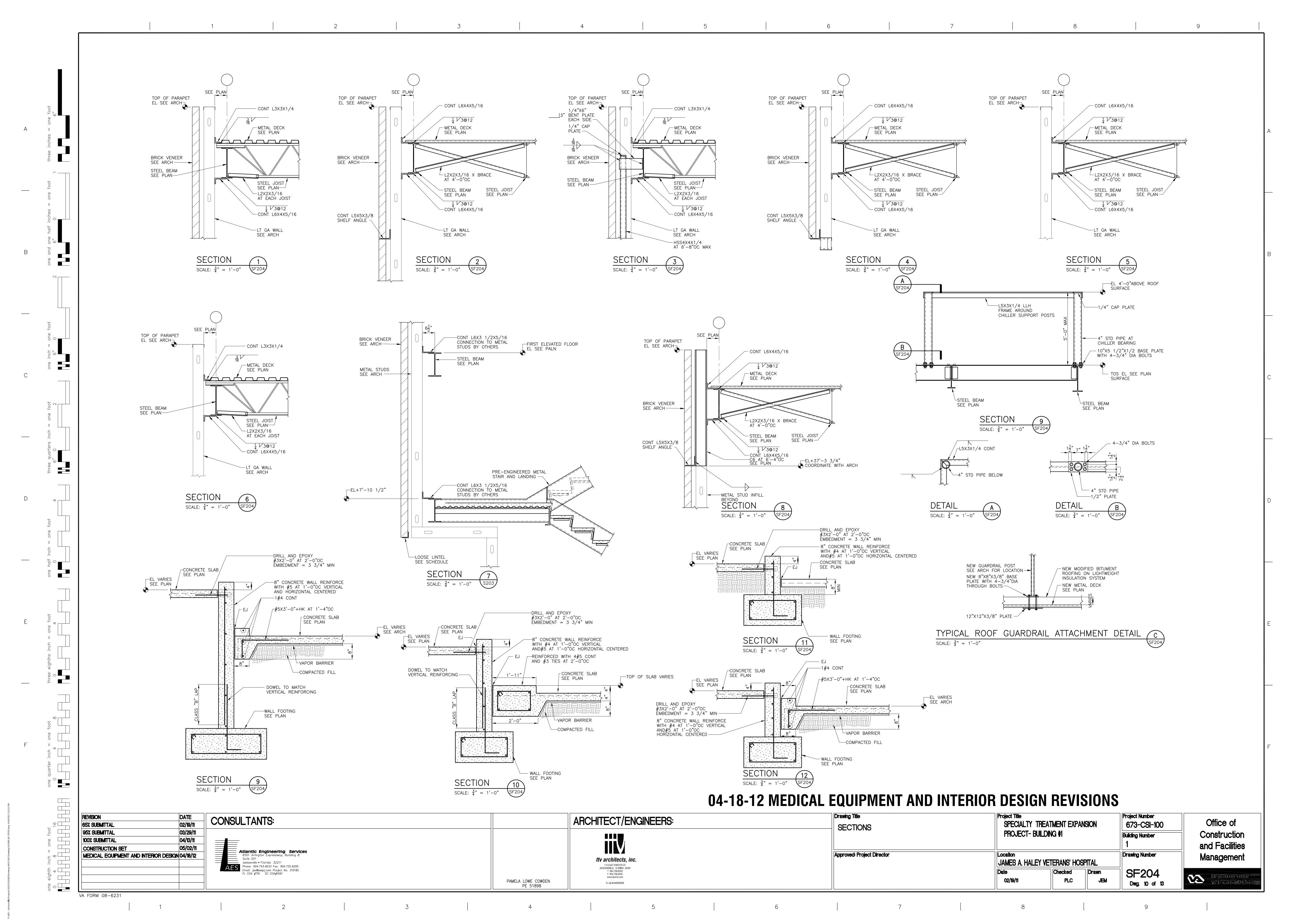
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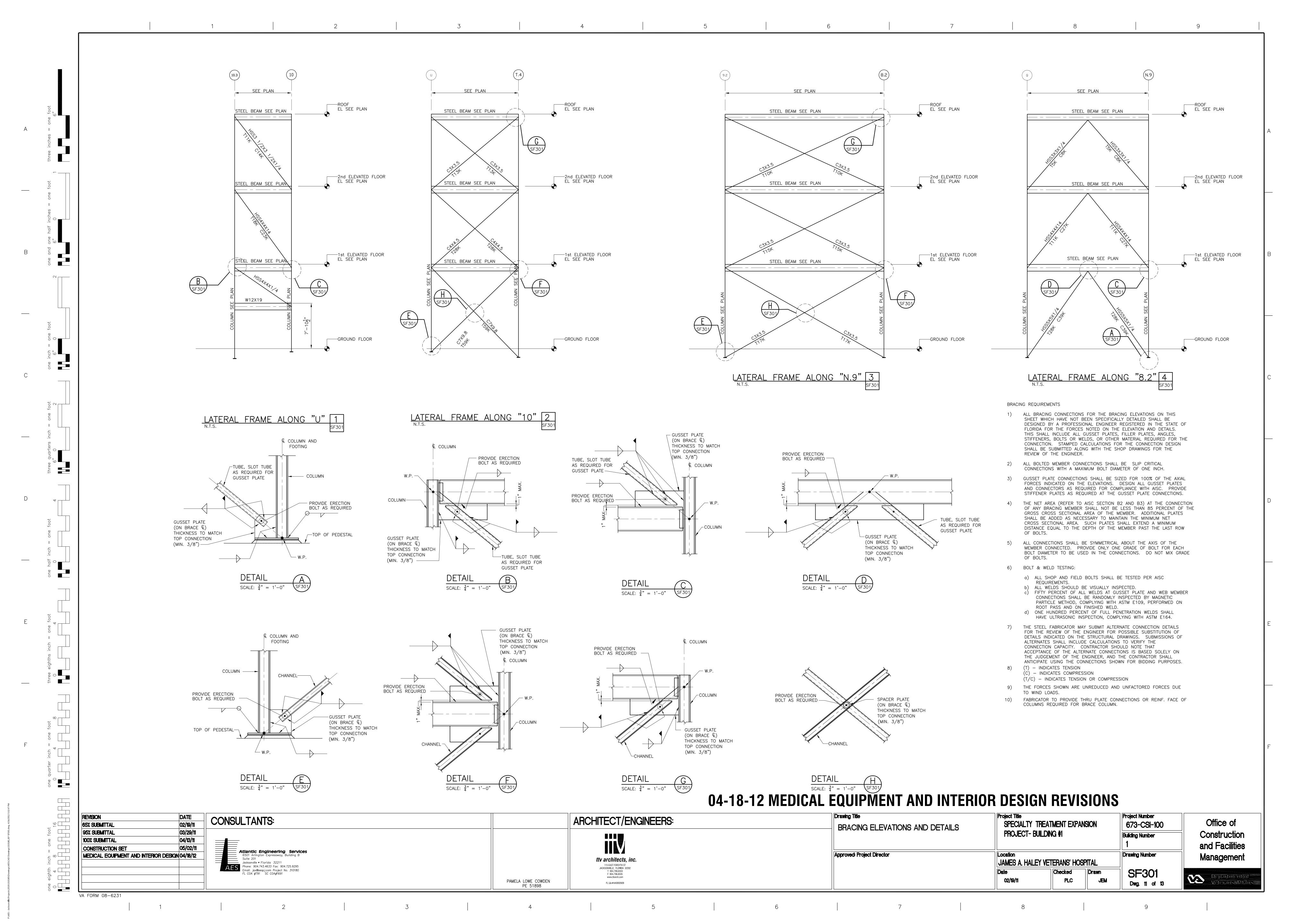
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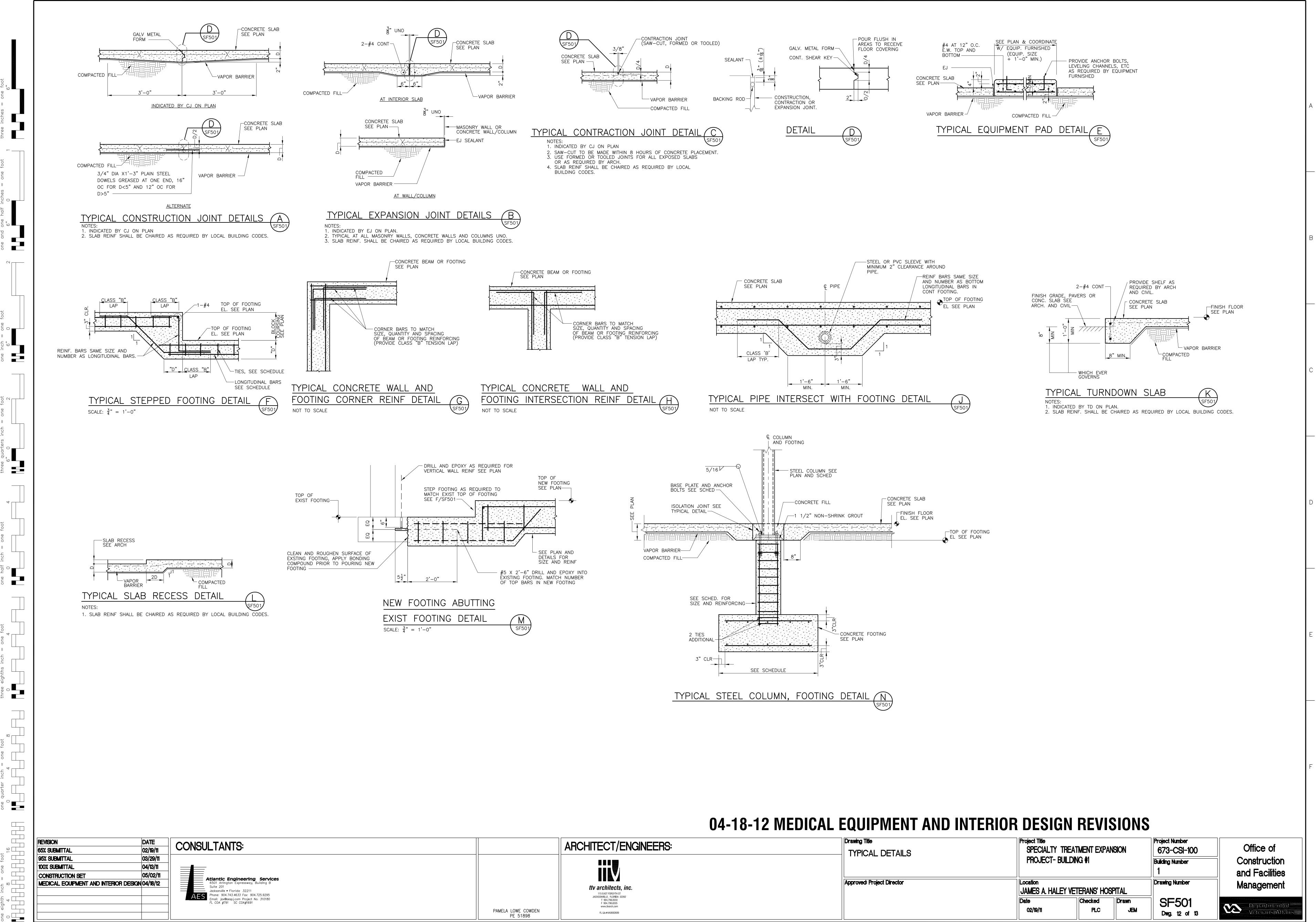












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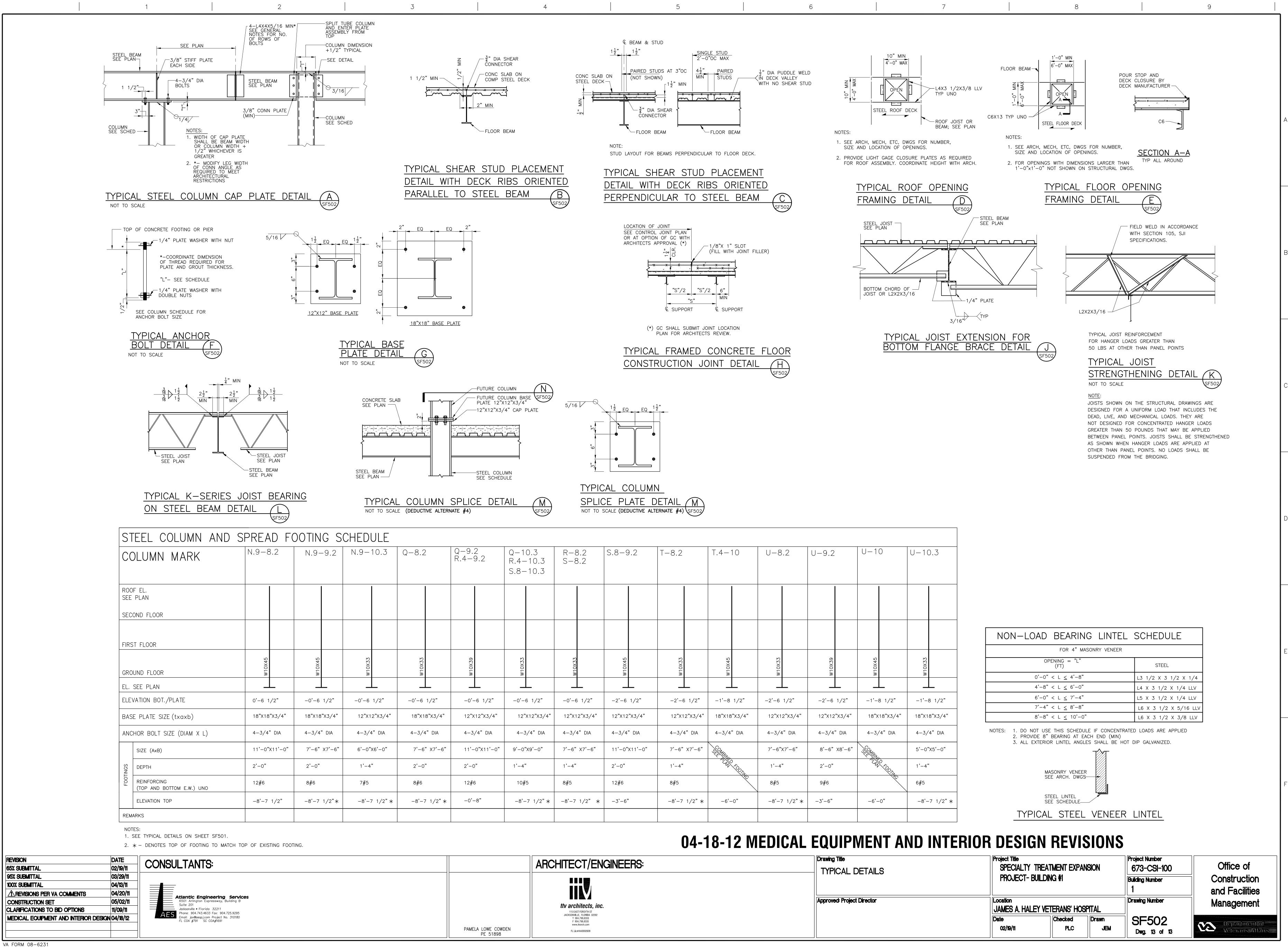
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